

Theoretical and experimental analysis of multifunctional high performance cement mortar matrices reinforced with varying lengths of carbon fibers

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ABSTRACT: An effective scheme to formulate high performance and multifunctional cement based mortar composites reinforced with varying lengths of carbon fibers has been devised. The detailed investigations pertaining to the fracture response of composites in cracks initiation and progression phases, their conducting mechanism and volumetric stability were performed with varying loads of 6mm and 12mm long carbon fibers at two different w/c ratios i.e. 0.45 and 0.50. The experiments concluded that an optimum addition of carbon fibers results in substantial improvement of fracture properties alongside significant reduction in electrical resistivity and total plastic shrinkage. The field emission scanning electron microscopy of the cryofractured specimen revealed crack arresting actions of uniformly distributed carbon fibers through successful crack bridging and branching phenomenon.

KEYWORDS: Fiber reinforcement; Mortar; Flexural strength; Compressive strength; Microcracking

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RESUMEN: *Análisis teórico y experimental de matrices multifuncionales de mortero de cemento de alta resistencia reforzadas con fibras de carbono de distintas longitudes.* Se describe un sistema eficaz para la elaboración de morteros de cemento compuestos de alta resistencia, multifuncionales y reforzados con fibras de carbono de 6 mm y de 12 mm de longitud. Con este trabajo pormenorizado se pretende determinar, en morteros reforzados con fibras de carbono y con relaciones a/c de 0,45 y 0,50: i) la resistencia a la rotura bajo distintas cargas en las fases de iniciación y propagación de fisuras; ii) el mecanismo responsable del agrietamiento; y iii) la estabilidad volumétrica. Los resultados experimentales demuestran que la incorporación de una proporción óptima de fibras de carbono conlleva una mejora considerable de la resistencia a la rotura, además de una importante reducción tanto de la resistividad eléctrica como de la retracción plástica total. El análisis por microscopía electrónica de barrido de emisión de campo de las probetas criofracturadas puso de manifiesto la capacidad de las fibras de carbono, homogéneamente distribuidas, de detener el agrietamiento mediante el puentado de las fisuras y el proceso de la ramificación.

PALABRAS CLAVE: Refuerzo de fibras; Mortero; Resistencia a la flexión; Resistencia a la Compresión; Microfisuración

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1. INTRODUCTION

Fiber reinforced cementitious composites (FRCCs) are gaining importance for their superior mechanical, electrical and chemical properties. Although the history of fibers utilization in composites dates back to 1500 BC but in recent years many new types of synthetic fibers have been developed and being utilized along with natural fibers such as polypropylene (PP), polyvinyl alcohol (PVA), steel, glass, carbon fibers (CFs) and carbon nanotubes (CNTs) (1–10). Among synthetic fibers, CFs possess exceptional physical and mechanical characteristics; therefore, they are being used as reinforcement to augment the mechanical properties (i.e. strength and ductility) of composites (11–17). Beside strength and ductility, the fibers also improve the durability characteristics of the cementitious composites by refining their microstructure and restricting the development of induced micro cracks due to shrinkage and other chemical/physical phenomenon (18–23). The mechanical and fracture behavior of FRCCs depend upon several factors such as fiber's volume, length, diameter, their dispersion and bonding with the host matrix. CFs of smaller lengths are easy to disperse as compared to the longer or continuous ones; but their alignment is of paramount importance in the attainment of high strength i.e. Hambach et al. reported an increase of 13.4 times in the flexural strength of cement composites by incorporating highly aligned CFs prepared by nozzle injection technique (24). The CFs in the cement composites improve their peak strength as well as the pre and post peak energy absorption capacities (25, 26). The increase in fracture energy and tensile strength of fiber reinforced cementitious matrix depends upon the content and aspect ratios of added fibers (27). Composites reinforced with longer fibers exhibit improved mechanical performance in terms of tensile strength with substantial decrease in the workability whereas, shorter fibers produce relatively lesser improvement in mechanical characteristics.

In cementitious composites, the unstable crack propagation starts with the development of first crack in the matrix whereas, in fiber reinforced cementitious composites the crack propagation is restricted by the fibers which act as bridge within the cracked portions of the matrix. Fibers bridging controls and hinders the unstable growth of the cracks thus imparting toughness and energy absorption capability to the cementitious composites (28, 29). CFs not only possess exceptional mechanical properties but also exhibit high electrical conductivity, therefore their inclusion in cement matrices induce multifunctional characteristics (i.e. high fracture energy, toughness, strength and electrical sensitivity) in the resulting composites. Several researchers have reported appreciable increase in the

mechanical performance of cementitious composites by incorporating fibers of various characteristics in terms of their type, shape, length, inclusion content etc. (30–45). However, the literature is deficient in clearly discussing the relationship between CFs aspect ratios and the fracture performance of cement composites in conjunction with their electrical properties. Therefore, by considering the strong influence of length to diameter ratios of CFs on the mechanical and electrical performance of cementitious composites it is considered imperative to carry out an in-depth investigation on these parameters. To achieve this, in the present research, the CFs having two different aspect ratios were incorporated in the cementitious mortar composites with w/c of 0.45 and 0.50 and detailed investigations were carried out to analyze the mechanical strength, crack propagation, fracture energy, fracture toughness, electrical response, volumetric stability and micro structural characteristics of the modified multifunctional cementitious matrices. Furthermore, the critical pullout length and percolation threshold of carbon fibers were evaluated using different theoretical approaches and then the analytical values were used to compare and validate the experimental results.

2. MATERIALS AND METHODS

2.1. Materials

In the present research work, Grade-53 ASTM Type-1 (46), ordinary Portland cement (OPC), commercially available under the brand name of “Bestway” conforming to ASTM C150 specifications was utilized. The chemical and physical characteristics of OPC are presented in Table 1. The well-graded siliceous sand with fineness modulus of 2.59 was used as fine aggregate. The polyacrylonitrile based CFs were acquired from Beijing Great Wall Co. Ltd. and used without any surface treatment. The detailed characteristics of CFs are reported in Table 2. Chemical grade Propane-1,2,3-triol (liquid glycerin) obtained from Sigma-Aldrich was used as a dispersant to prepare a homogenized suspension of carbon fibers before mixing them into the cement.

2.2. Dispersion of carbon fibers

The uniform dispersion of fibers in any fiber-reinforced matrix is crucial for achieving optimum performance. In cementitious composites, it is usually difficult to ensure thorough dispersion of fibers in the cement matrix. Therefore, special attention was paid to achieve excellent dispersion of CFs in water prior to their addition into the cement. Initially, the water was mixed with CFs and sonicated for 10 min via high-energy sonication probe (UP50H, operated at 90% amplitude). Then, 2.7% Propane-1,2,3-triol by weight of water was added in

TABLE 1. Chemical composition and physical properties of OPC

Chemical composition	CaO	SiO ₂	Al ₂ O ₃	Fe ₂ O ₃	MgO	SO ₃	Minor contents*
Content (% by mass of cement)	61.70	21.00	5.04	3.24	2.56	1.51	4.95
Physical property	Insoluble residue	Specific gravity	Specific surface area	Particle size (d ₅₀)	Loss on ignition		
Content	0.54 %	3.10	3.45 m ² /g	13.58 μm	1.83 %		

* Minor contents such as Na₂O, K₂O, P₂O₅ and TiO₂ etc.

TABLE 2. Physical characteristics of carbon fibers

Designation	Length (mm)	Diameter (μm)	Density (kg/m ³)	Purity (%)	Aspect ratio	Tensile modulus (GPa)	Tensile strength (MPa)
CF1	6.00	7.00	1.76	≥ 98	857	230	3450
CF2	12.00	7.00	1.76	≥ 98	1714	230	3450

to the suspension and again sonicated for 10 min. The Propane-1,2,3-triol assisted chemically to prepare homogeneous suspension of CFs in water. The dispersion level of CFs in water was analyzed through optical microscope (Figure 1).

2.3. Cementitious composite sample preparation

In total, five categories of cementitious mortar composite samples were prepared for each carbon fiber type with varying loads (i.e. 0.00, 0.05, 0.10, 0.15 and 0.20% by weight of cement) analyzed at 0.45 and 0.50 w/c ratio. The ratio of cement to sand was fixed at 1:1.3 for all the composite samples. The complete description of mortar composite samples is presented in Table 3. For the preparation of composite samples, the measured amounts of cement and sand were added to the 9.0 L Hobart mixer and dry mixing was carried out for 2 min. After dry mixing, the required amount of water having thoroughly dispersed CFs was added and mixing was continued for 6 min at high shear rate. Then, the mixture was poured in to the stainless steel molds of 40×40×160 mm³ dimensions. The molds

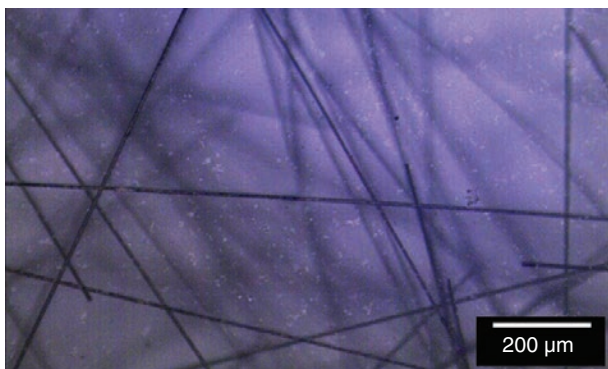


FIGURE 1. Optical microscopic image of dispersed 6 mm fibers (CF1) in water

were wrapped in plastic sheets and left at room temperature (i.e. 20.0°C and 95% relative humidity) for 24 h. The dried samples were then removed from the molds and immersed in water and curing was carried out till the desired age of testing.

2.4. Characterization of composite samples

The cement mortar composite samples were analyzed for their flexural and compressive strengths by using universal testing machine (Pneumatic IPC Global UTM-25P) at 7, 14 and 28 days of curing.

TABLE 3. Composition of cementitious mortar formulations

Notation	W/C ratio	Carbon Fiber Type	Carbon Fiber Content (%)
45M	0.45	-	0
45MCF105	0.45	CF1	0.05
45MCF110	0.45	CF1	0.10
45MCF115	0.45	CF1	0.15
45MCF120	0.45	CF1	0.20
45MCF205	0.45	CF2	0.05
45MCF210	0.45	CF2	0.10
45MCF215	0.45	CF2	0.15
45MCF220	0.45	CF2	0.20
50M	0.50	-	0
50MCF105	0.50	CF1	0.05
50MCF110	0.50	CF1	0.10
50MCF115	0.50	CF1	0.15
50MCF120	0.50	CF1	0.20
50MCF205	0.50	CF2	0.05
50MCF210	0.50	CF2	0.10
50MCF215	0.50	CF2	0.15
50MCF220	0.50	CF2	0.20

*For each mix 750 g cement, 975 g sand and 2.7% Propane-1,2,3-triol by weight of water was used.

The prism samples were tested under three-point bending in displacement controlled mode with a displacement rate of 0.005 mm/min as shown in Figure 2a, while the two broken halves were analyzed for their compressive strength at a controlled rate of 1.2 mm/min. Electrical resistance values of the composite specimens were tested by using digital multi-meter (U1733C, Agilent Technologies, Inc., USA) having 0.20% of measuring accuracy via two electrode method (Figure 2b). For carrying out the electrical resistivity measurements, electrical

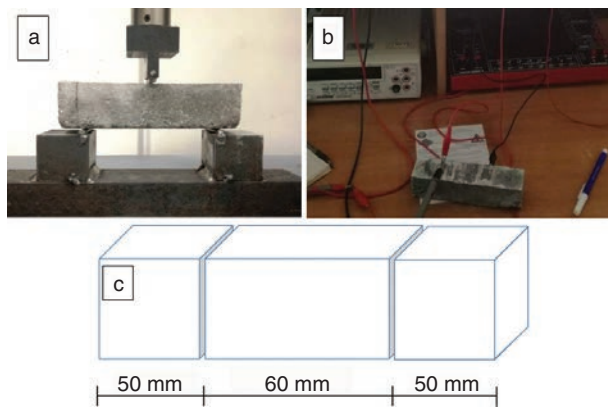


FIGURE 2. Experimental setup for three-point bending (a), electrical resistivity measurements (b) and electrical contact configuration (c)

conducting paint was applied around the perimeter of the samples at specified planes (Figure 2c). The painted planes were wrapped with copper wires to ensure perfect contact between the sample and the electrodes. To study the influence of carbon fibers addition on the shrinkage behavior of mortar composites, the specimens were tested using shrinkage apparatus (Schwindrine, Germany) operated under linear protocol. Small chips of the broken samples were analyzed through scanning electron microscope (SEM) to observe the attained level of dispersion of CFs, their morphology and interfacial interaction with the host cement matrix.

3. RESULTS AND DISCUSSIONS

3.1. Dispersion of carbon fibers in cement composites

The dispersion of CFs is very crucial and plays an important role in crack bridging and fracture energy enhancement of the composites. The dispersion was thoroughly analyzed by SEM analysis; typical micrographs showing the dispersion of CF1 and CF2 in cement mortar composites are presented in Figure 3. The observations indicate that the selected dispersant, dispersing scheme and mixing regime are very effective in achieving the thorough dispersion of CFs inside the host mortar matrix. In the cement mix, the surfactant reduces the surface tension thus effectively restricting the negative

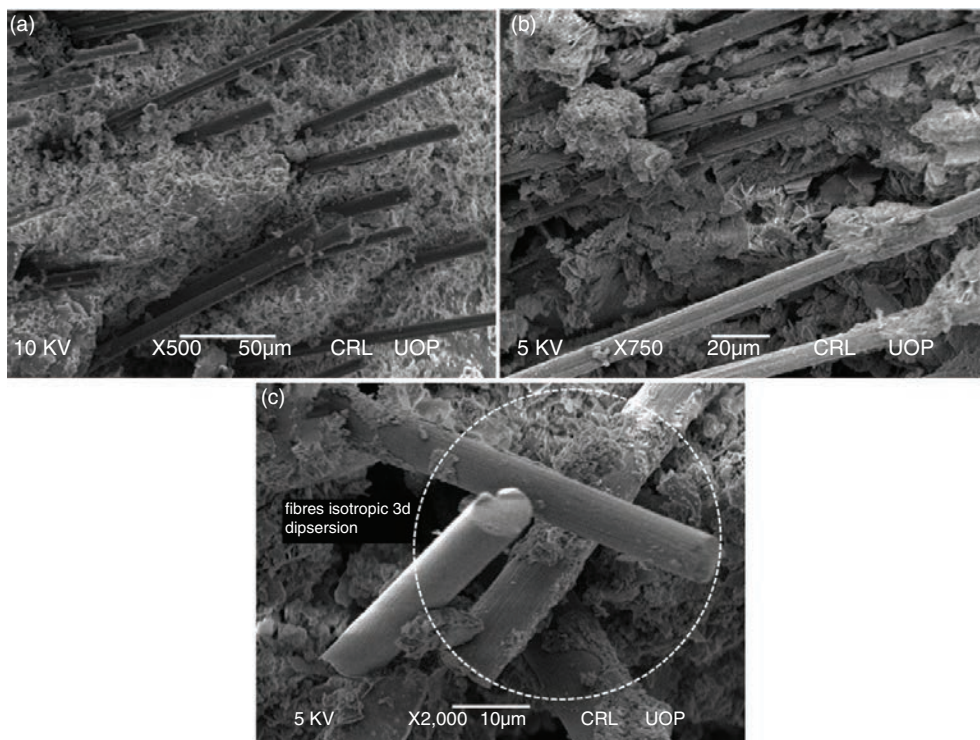


FIGURE 3. FESEM micrograph showing dispersion of CFs in cement matrix (a) CF1, and (b & c) CF2

capillary pressures. This effect results in availability of more free water for providing the workability and thorough dispersion of carbon fibers in the mix. Additionally, this also reduces the overall shrinkage of the cement paste as demonstrated by several researchers (47, 48).

3.2. Flexural behavior of cement mortar composites

The typical load vs displacement behavior of the cement mortar composites samples after 28 days of curing is presented in Figure 4. The flexural test results reveal that the addition of CFs not only enhances the peak stress but also increases the rupture strain.

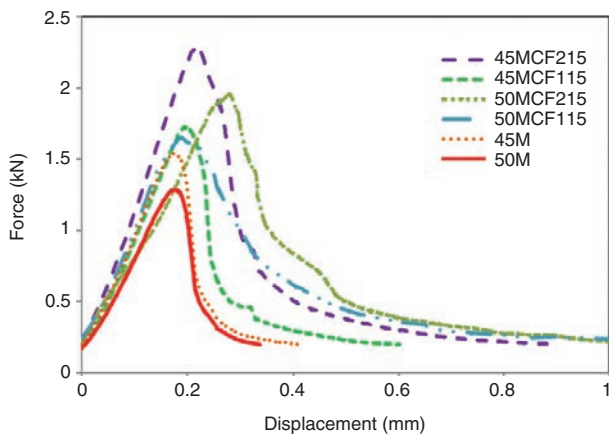


FIGURE 4. Typical force vs mid-point displacement curves for CFs modified cement mortar composites at 28 days

The addition of CF1 and CF2 by 0.15 wt% at w/c of 0.45 exhibited 32% and 83% increase in modulus of rupture (MOR), respectively. The carbon fibers inclusion also increases the ductility factor (μ), which is defined as the difference between the ultimate displacement (δ_u) and the displacement at yield (δ_y). The influence of various amounts of carbon fibers on the ductility factor is presented in Figure 5. The MOR of cement mortar specimens was determined using eq. [1] under three-point bending test. Here, ‘P’ is the maximum force in the load deflection curve, ‘l’ is the span length, ‘w’ is the specimen width and ‘h’ is the specimen height [1] (49):

$$MOR = 3P l/2wh^2 \quad [1]$$

The comparative analysis reveals that the MOR of cement mortar composites initially increases with the CFs additions up to 0.15 wt% and afterwards the trend begins to decline as reflected in Figure 6. The decrease in MOR with higher amount of CFs may be associated to the inter-fibers slippage and lesser effective dispersion due to the presence of relatively high number of fibers. The carbon fibers with higher aspect ratio performed better as compared to the shorter ones because the longer fibers possess more chance to bridge the growing crack in the composites due to their relatively longer lengths.

The failure deformation results indicate that the higher fiber content results in more capacity of cement mortar composites to undergo deformation before ultimate fracture. However, the total energy

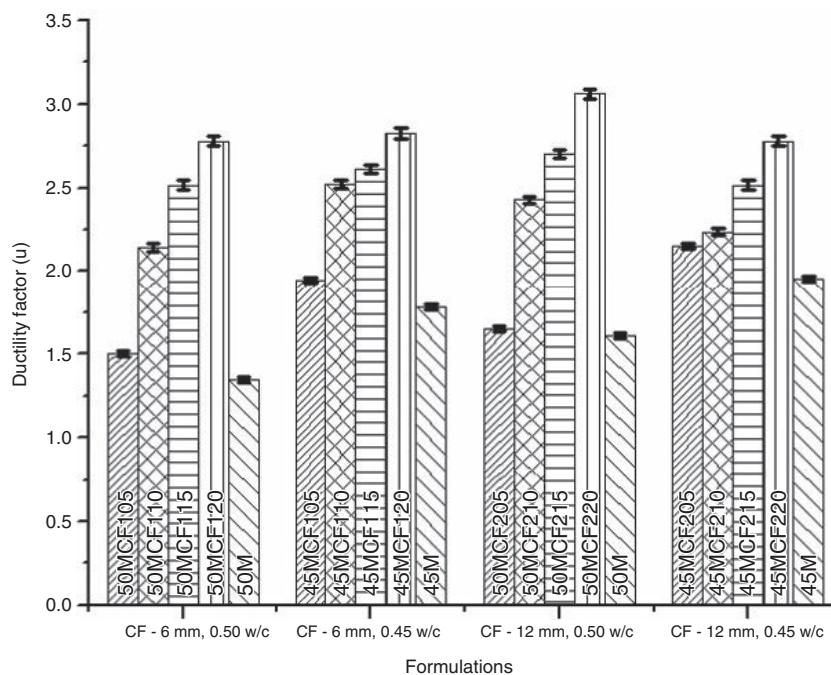


FIGURE 5. Ductility factor of the CFs modified cement mortar composites

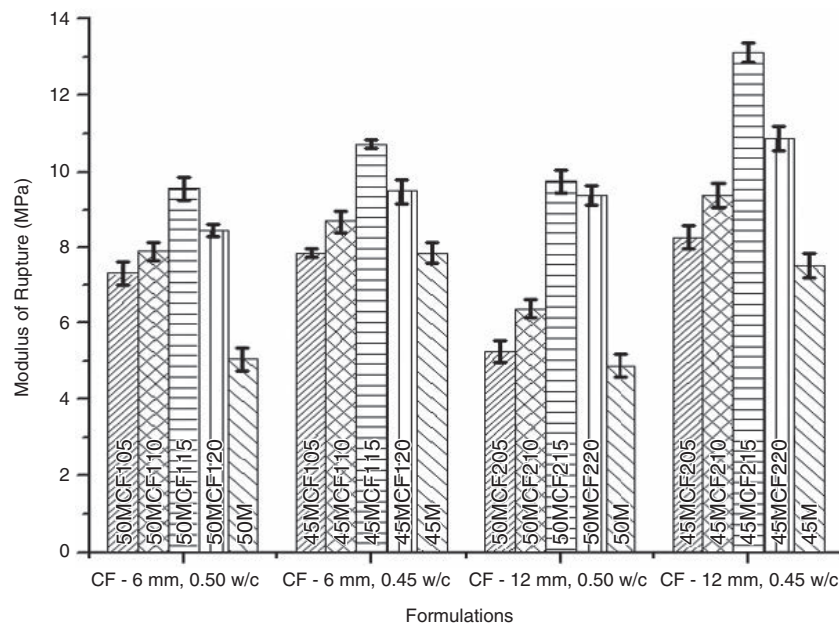


FIGURE 6. Modulus of rupture of the CFs modified cement mortar composites

absorption indicate that an optimum level of inclusion exists after which the performance of the resulting composites flattens as demonstrated through Figure 7. Hambach et al. (2016) reported the flexural strength enhancement of 5.5 and 14.3 times with the inclusion of perfectly aligned 1.0 and 3.0% carbon fibers in the cement paste (24), indicating that the flexural strength enhancement is not only the function of fibers content but also their orientation in the matrix (50, 51).

The absolute values of toughness (energy absorbed during fracture) depends entirely upon the specimen geometry and testing arrangements. Therefore, to enable realistic comparison of performance, ASTM C 1018 (52) suggests to evaluate I_5 , I_{10} and I_{20} , defined as the ratios of area under the load-CMOD curve up to the deflections of 3.0, 5.5 and 10.5 times the first-crack deflection to the area up to first-crack, respectively (53, 54). The toughness indices I_5 , I_{10} and I_{20} has been calculated indicating the post crack toughness

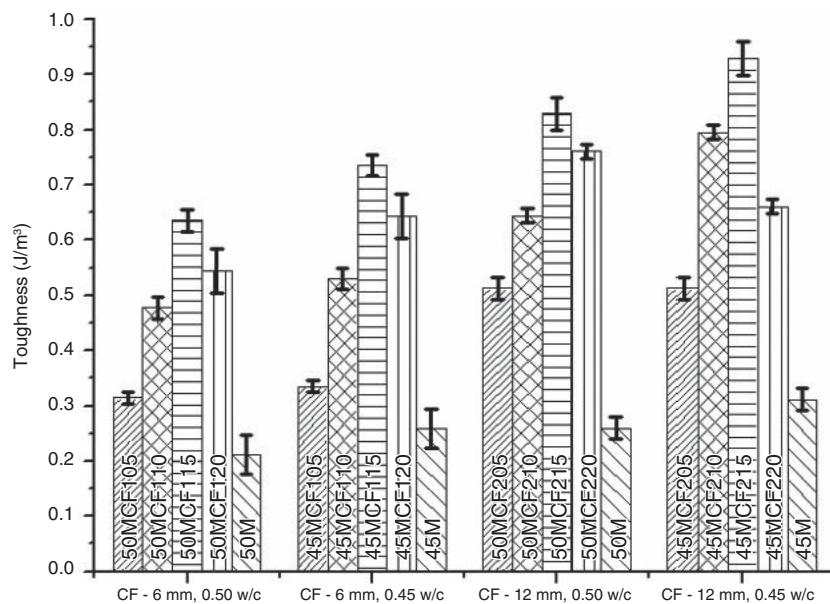


FIGURE 7. Ultimate toughness comparison of CFs modified cement mortar composites

behavior of the carbon fiber reinforced prisms and presented in Figure 8.

Residual strength factors (RSFs), which are derived directly from toughness indices, characterize the level of strength retained after first crack simply by expressing the average post-crack load over a specific deflection interval as a percentage of first crack load (52). Trend attained against the varying lengths of reinforcing carbon fibers with the optimum additions is shown in the scatter given in Figure 9; mathematically RSFs are defined as follow [2] [3]:

$$R_{(5,10)} = 20 * (I_{10} - I_5) \quad [2]$$

$$R_{(10,20)} = 10 * (I_{20} - I_{10}) \quad [3]$$

Based on above experimentally evaluated values, percentage increase in toughness indices and residual strength factors with 6 mm and 12 mm long CFs are summarized in Table 4.

3.3. Behavior of cement mortar composites in compression

The compressive strength test results indicate that the addition of CFs has slight influence on the compressive behavior of modified formulations as shown in Figure 10. The slight decrease at higher level of CFs additions may be attributed to the induced weakening effect of the entrained airvoids. In general, CF1 produced better compressive strength as compared to CF2. This effect may be attributed to the more efficient dispersion of short fibers in the matrix as compared to the relatively

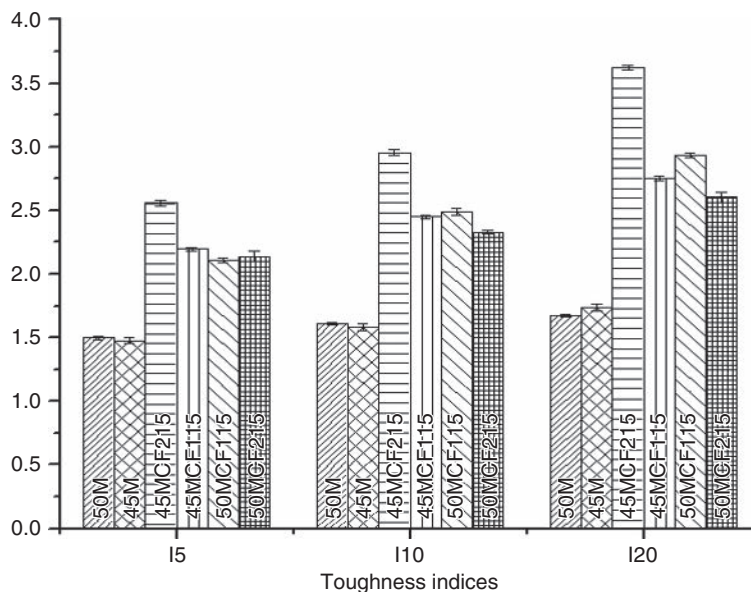


FIGURE 8. Comparison of the toughness indices of CFs modified cement mortar composites

longer fibers. Figure 11 presents the comparison of compressive strength for 45MCF15 at 7, 14 and 28 days. The reported increase seems to decline with the increasing age of cementitious mortar that might be associated to the much refined microstructure of the reference mix when reaching the characteristic age of 28 days.

3.4. Electrical resistivity of carbon fiber reinforced cement mortar composites

The electrical resistivity results of CF1 and CF2 filled cement mortar composites with w/c of 0.45 are reported in Figure 12. The electrical resistivity of cement mortar sharply decreases as the content

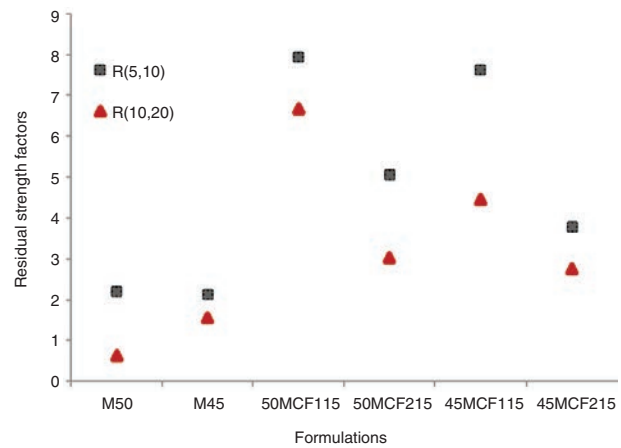


FIGURE 9. Residual strength factors of CFs modified cement mortar composites

of conducting carbon fibers in the matrix increases (55, 56). An occurrence of enormous decrease in the value of resistivity at 0.15 wt% inclusion of 12mm carbon fibers, is the sign of establishment of conducting mechanism inside the cementitious matrix due to the induced carbon fibers. An overall decrease in the electrical resistivity by 57.1% and 67.8%, using 6 mm and 12 mm long carbon fibers evidences that the conductive mechanism of long fibers filled cement mortar is better than that of the shorter one.

3.5. Volumetric stability

Due to the reinforcing action, addition of CFs in the matrix tends to reduce the formation of cracks and thereby improve the shrinkage response of the

TABLE 4. The increased rate of toughness indices and residual strength factors

Samples	% increase in toughness indices			% increase in residual strength	
	I ₅	I ₁₀	I ₂₀	R _(5,10)	R _(10,20)
M50	100.0	100.0	100.0	100.0	100.0
50MCF115	170.6	183.5	216.4	358.1	1059.6
50MCF215	146.5	152.1	164.4	228.9	379.5
M45	100.0	100.0	100.0	100.0	100.0
45MCF115	142.8	157.3	168.9	358.2	287.2
45MCF215	144.8	147.0	149.7	178.0	177.4

resultant formulations. In total, six formulations were selected to study the shrinkage response and their results are plotted in Figure 13. The results depicted significant reduction in the values of shrinkage with the proportionate increase in the content and the length of added carbon fibers. Maximum reduction of 307% as compared to the reference specimen was observed for 0.20 wt% addition of CF2 (Figure 14). Researchers have shown that free shrinkage and early age cracking of cementitious composites may be effectively controlled by using

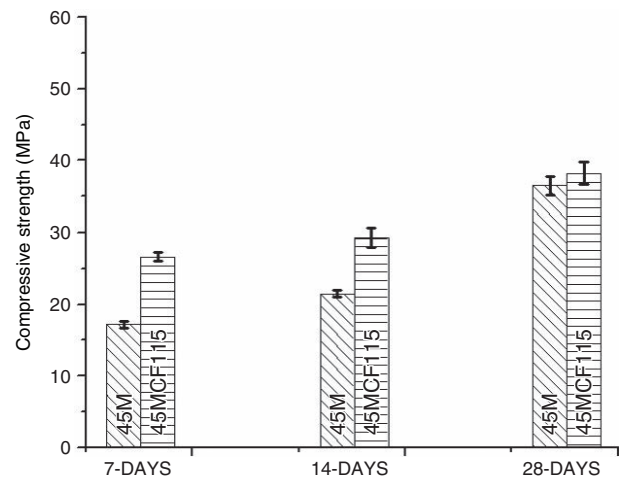


FIGURE 11. Comparison of effect of age for CFs modified mortar composites having optimum performance with the reference mix

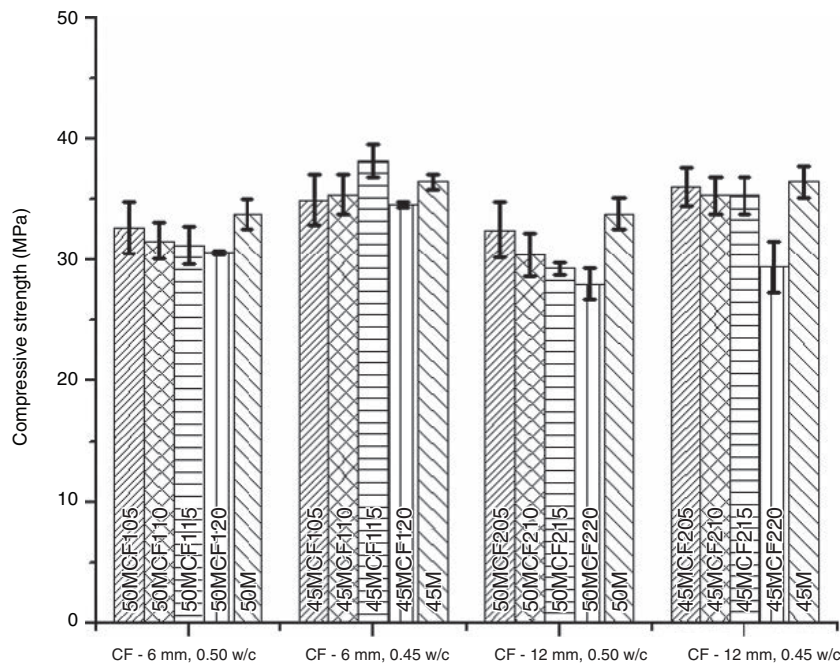


FIGURE 10. Comparison of characteristic compressive strength of CFs modified cement mortar composites at 28-days

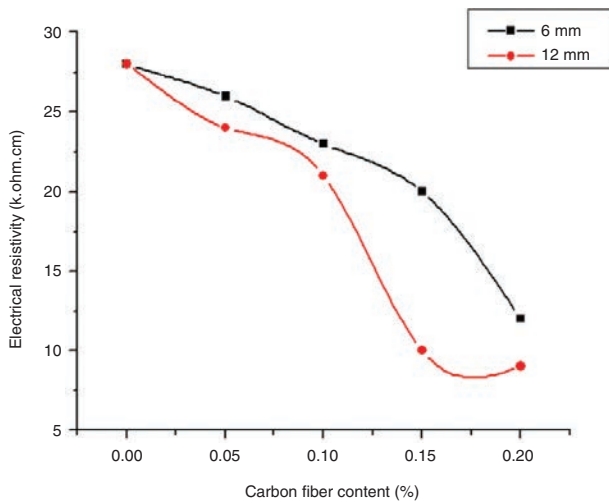


FIGURE 12. Comparison of the electrical resistivity of 6 mm and 12 mm carbon fiber filled cement mortar

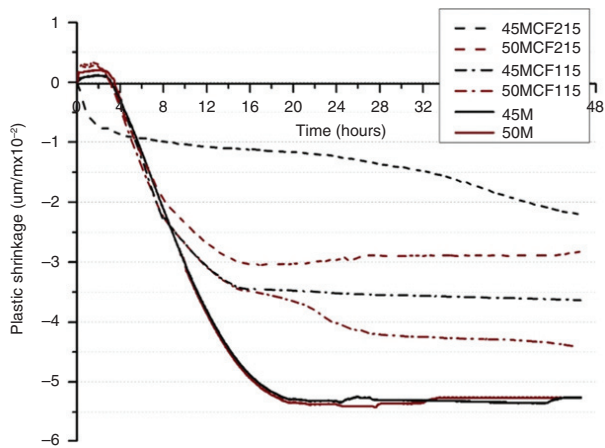


FIGURE 13. Comparison of the total linear shrinkage of 6 mm and 12 mm long carbon fiber filled cement mortar

propylene or sisal fibers possessing very low elastic modulus as compared to the carbon fibers (57,58). This phenomenon is regarded to the low modulus of cement composite at early ages that is usually low as compared to the strength and elastic modulus of fibrous inclusions (59,60).

3.6. Microstructure

The hardened cement mortar samples were observed under scanning electron microscope to further elaborate the strengthening mechanism of carbon fibers in the composites. The micrographs displayed evident signs of crack bridging and branching phenomenon along with fiber breakage and pullout at some occasional sites. The strength of interfacial bond between carbon fiber with cementitious matrix may be justified by the lightened groove marked on

the matrix after the pull out of fiber under critical stress (Figure 15a).

3.7. Theoretical calculations and analysis

To understand the mechanics involved in the fracture behavior of carbon fiber reinforced cement mortar and to have the analytical validation of performed mechanical and electrical tests, some theoretical approaches were used in the form of mathematical equations. In the initial phase, numbers of carbon fibers were estimated according to eq. [5] that might be expected in unit volume of cementitious mortar as plotted in Figure 16. Then using the proposed equation by Romualdi et al. (61), the average center to center distance between the adjacent fibers was determined according to eq. [6], presented in Figure 17. In the end critical pull-out length was evaluated by relating the tensile and flexural strength of reinforced cement based mortar composite as established by Hannanat et al (62). The calculated critical pullout length according to eq. [7] is presented in Figure 18. In the calculation procedure, the volume fraction of carbon fiber ‘V_f’ can be expressed as [4]:

$$V_f = \frac{V}{V_{fc}} = \frac{m}{\rho} \cdot V_{fc} \quad [4]$$

Here, ‘V’ is the total volume of carbon fiber in mm³, ‘V_{fc}’ is the volume of carbon fiber filled cement mortar composite i.e. 40×40×160 mm³, ‘ρ’ is the density of carbon fiber i.e. 1800 kg/m³ (as per manufacturer product data sheet) and ‘m’ is the mass of carbon fiber. The number of carbon fibers ‘N’ enclosed in a cubic millimeter of carbon fiber filled cement based composite can be estimated by the following expression [5]:

$$N = \frac{V_f}{A_f L_f} = \frac{V_f}{\pi \left(\frac{d_f}{2}\right)^2 \times L_f} \quad [5]$$

Here, ‘A_f’ is the cross-sectional area of carbon fiber in mm², ‘L_f’ is the length of carbon fibers in mm and ‘d_f’ is the diameter of carbon fiber i.e. 7×10⁻³ mm. The average center-to-center spacing ‘s̄’ can be expressed as [6]:

$$\bar{s} = 13.8 \times d_f \times \sqrt{\frac{1}{V_f}} = 0.6557 \times \sqrt{\frac{1}{m}} \quad [6]$$

Assuming carbon fiber is snapped at half of its length then critical pull out length ‘L_f^{crit}’ can be estimated using following equation [7]:

$$L_f^{crit} = L_f \times \sqrt{\frac{\sigma_f^u \eta_o V_f}{2\sigma_{fc}^u}} \quad [7]$$

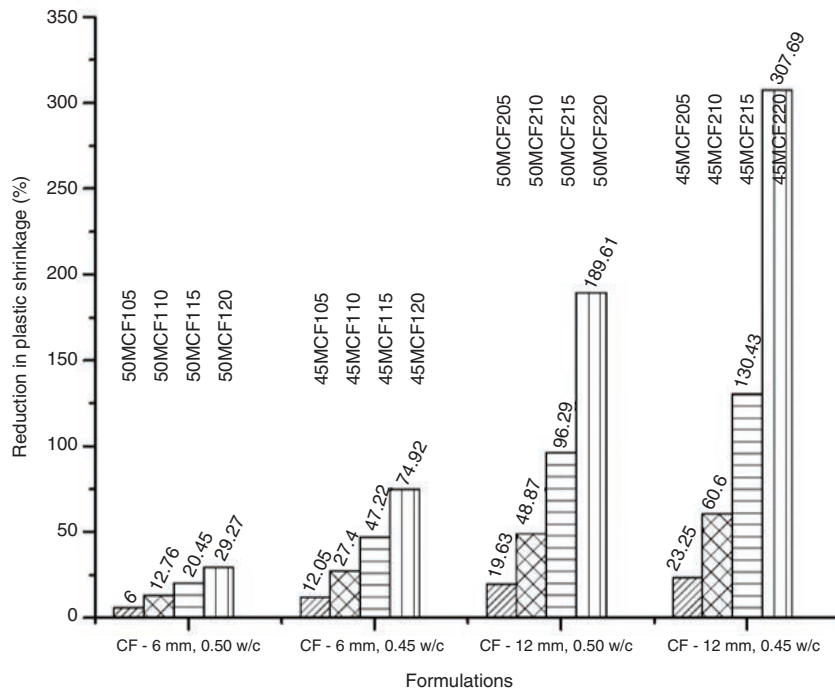


FIGURE 14. The decrease rate in the total linear shrinkage of carbon fiber filled cement mortar specimens

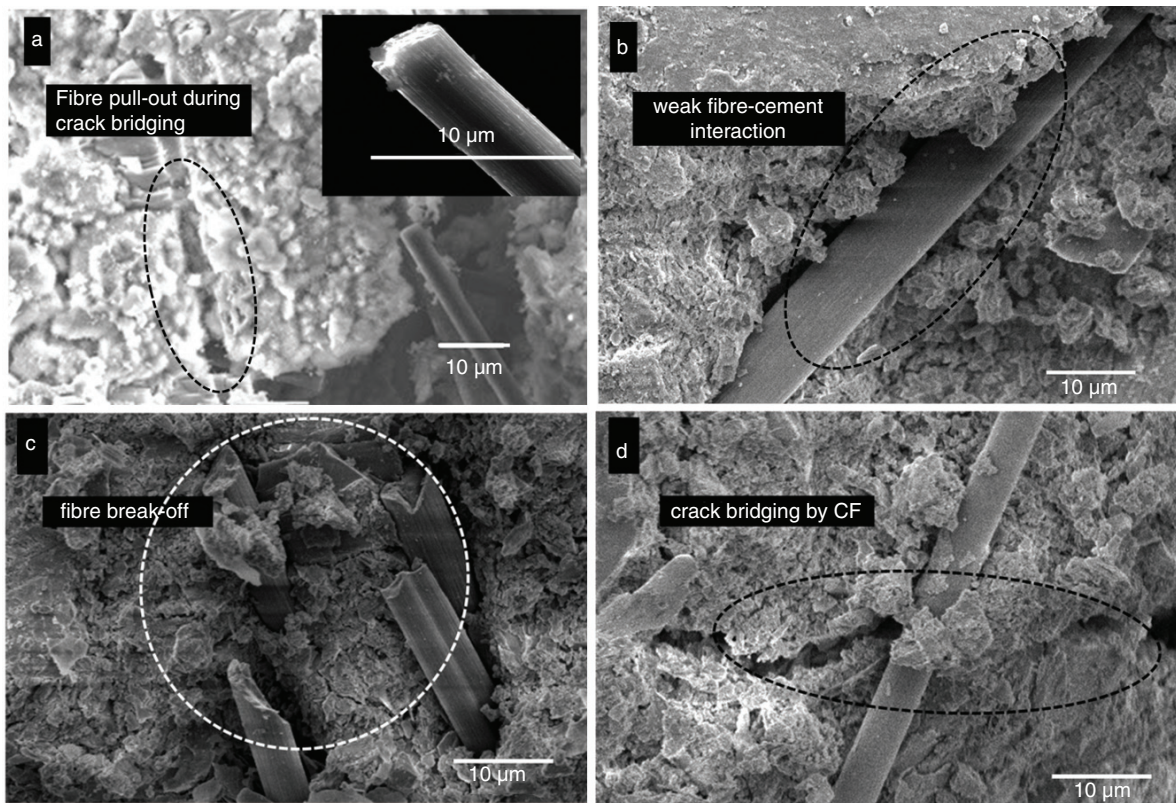


FIGURE 15. FESEM micrographs evidencing strengthening mechanism of cement mortar composite samples reinforced with CFs

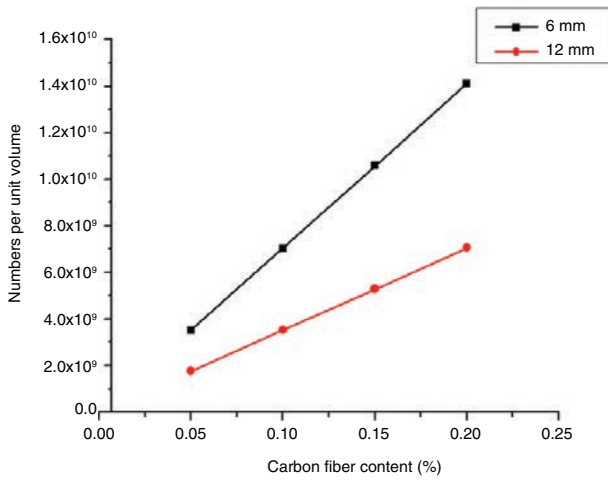


FIGURE 16. Estimation of the number of carbon fibers encapsulated inside the unit volume of carbon fiber filled cement mortar specimens (61)

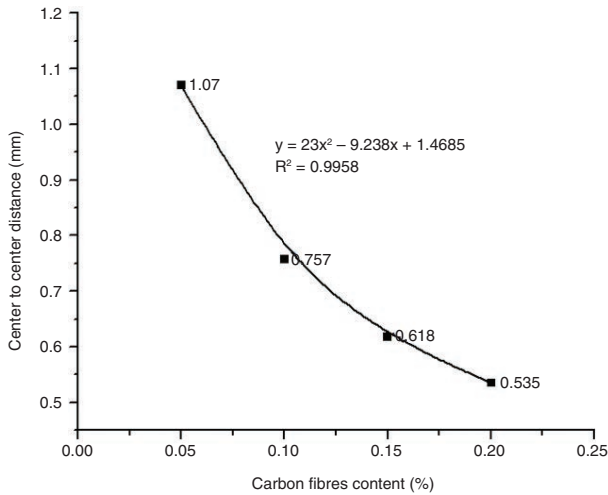


FIGURE 17. Average center-to-center spacing between two carbon fibers in carbon fiber filled cement mortar specimens calculated according to eq. [6] (62).

Here, σ_f^u is the tensile strength of carbon fibers i.e. 3450 MPa (as per manufacturer product data sheet), η_0 is the coefficient which depends upon the fibers orientation in the composite matrix and σ_{fc}^u is the tensile strength of carbon fibers filled cement mortar composite in MPa. The tensile strength of the composite σ_{fc}^u is associated with the flexural strength of the corresponding composite σ_{fc}^b evaluated through experiments by using the mathematical relation given in eq. [8].

$$\sigma_{fc}^u = \frac{\sigma_{fc}^b}{2.44} \quad [8]$$

Here, σ_{fc}^b is the 28 days flexural strength of carbon fiber filled cement mortar samples in MPa.

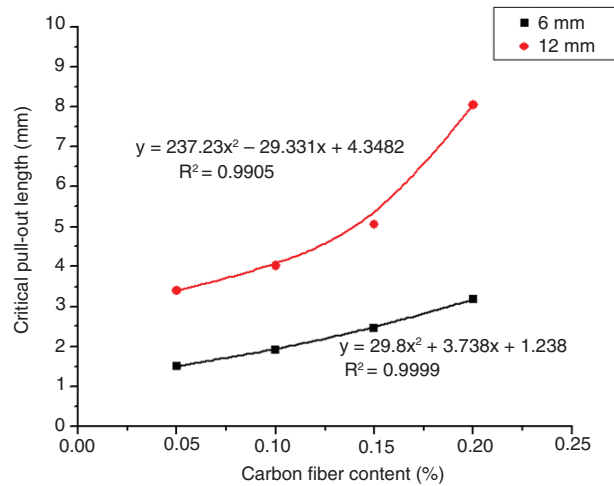


FIGURE 18. Estimated critical pullout lengths of 6 mm and 12 mm long carbon fibers calculated according to eq. [7].

TABLE 5. Theoretical percolations of 6 mm and 12 mm long carbon fibers

Percolation threshold (wrt wt% of cement)	IPD approach	Celzard approach (64)	
		Lower limit	Upper limit
CF1-6 mm; AR* = 857	0.0575	0.0029	0.0048
CF2-12 mm; AR* = 1741	0.0281	0.0015	0.0028

*AR (Aspect ratio)

For the conducting mechanism, Jing li et. al. (63) developed an improved analytical model based on IPD (Inter particle distance) to predict the percolation threshold 'P_c' (%) of graphite nano platelets with high aspect ratio in the polymer composites. His proposed equation [9] is as follows and the computed values for the present study are presented in Table 5:

$$P_c = \frac{27\pi d_f^2 L_f}{4(d_f + IPD)^3} \quad [9]$$

According to quantum tunnelling mechanism electron hopping occurs when IPD is less than or equal to 10 nm. Since IPD is very small in comparison with the average effective diameter 'd_f' of graphite nano platelets so it may be ignored and finally eq. [9] modifies in to eq. [10] given as:

$$P_c = \frac{27\pi L_f}{4d_f} \quad [10]$$

$$P_c = \frac{21.195}{\alpha} \quad [11]$$

$$\alpha = \frac{d_f}{L_f} \quad [12]$$

Celzard. et al. (64) proposed a relation between percolation threshold and aspect ratio of fillers based on the concepts of excluded volume; mentioned in eq. [13] (refer Table 5):

$$P_c = 1 - \exp\left(-\frac{3.6d_f}{\pi L_f}\right) \leq \phi_c$$

$$\leq 1 - \exp\left(-\frac{5.6L_f}{\pi d_f}\right) \quad [13]$$

3.8. Comparison of analytical and experimental results

The analytical results indicate that the number of carbon fibers in a unit volume increase linearly with the inclusion content but their center-to-center distance follows an exponential decremented pattern (Figure 16 and Figure 17 refers). The flattening pattern of center-to-center distance is more pronounced after 0.15 wt% inclusion of CFs. This phenomenon may be associated with the clumping and bundling of carbon fibers at higher concentrations (18). The clumping of fibers leads to heterogeneity inducing weak spots in the cementitious matrix. Therefore, an optimum enhancement in the fracture properties of the reinforced cement based mortar composite was observed with an addition by 0.15 wt% to cement.

The maximum length of carbon fiber to be pulled out from the matrix in damaged span defines the critical pull out length. The value of critical pullout length increases with proportionate increase in the content and length of carbon fiber. The 12 mm long, carbon fibers need more critical pullout length as compared to 6 mm long fibers for the same amount of inclusion (Figure 18 refers). The range of critical pullout length of carbon fibers inside cement mortar composites varies from 1.50 mm to 8.05 mm. In the matrix, if the embedment length of a fiber is shorter than its critical pullout length then the fiber will be snapped out from the cement matrix during fracture, as observed in Figure 15. At higher concentrations, most fibers were observed to be pulled out while observing the microstructure of the tortured specimens. An increase in the critical value of pullout length and weak CFs-cement matrix interaction may clearly justify such failure mechanism. At smaller concentrations, fibers are more effectively bonded with the host matrix and thereby use much of their capacity in loads transmittance as evident through the lower number of pullout observed in SEM micrographs of chipped specimens.

The electrical resistivity values were found to decrease with the added content of conducting reinforcements in the form of carbon fibers. The major contributing parameters towards conductivity of the resultant cement mortar composites are carbon fibers and the electrolyte ions. With proportionate increase in the content of carbon fibers,

their inter-fiber spacing begins to decline as shown in Figure 17. When the spacing reduces beyond certain limit that can entertain electrons from fiber to fiber to establish the tunnel effect, stable conductive mechanism is developed. It can be seen that the electrical resistivity decreases as the carbon fiber content increases until some optimum value after that the trend begins to flatten.

The percolations calculated via IPD (63) and Celzard (64) approaches were found to be underestimated on comparison with the values determined through experimentations. In fact, it is not possible to distribute and align all the fibers in their ideal 3-d orientations therefore, it is almost impossible to imagine the same values of theoretical percolations through experiments. Additionally, there are a few fibers in the state of lapping which adds to the content of experimental percolations. At higher concentrations it is relatively tough to efficiently and isotropically disperse all the fibers and the level of complexity further increase while dealing with the relatively long fibers.

4. CONCLUSIONS AND RECOMMENDATION

In this study, 6 mm and 12 mm carbon fibers were investigated to formulate high performance and multifunctional cement based mortar composites at the selected water to cement ratio of 0.45 and 0.50. After in-depth analysis of theoretical and experimental results pertaining to the fracture behavior, electrical properties and the microstructure, following conclusions may be drawn:

1. The inclusion of well dispersed carbon fibers in the cement mortar matrix not only enhance the modulus of rupture but also substantially improve the toughness and post crack performance.
2. An enormous improvement of about three times in ultimate toughness is achieved with an optimum addition of 0.15 weight percent of 6 mm and 12 mm long carbon fibers. Beyond the optimum content the performance tends to decline that may be associated to the clumping and bundling of carbon fibers at higher concentrations.
3. The decrease in electrical resistivity is relatively more for mixes containing 12 mm long carbon fibers. The percolation threshold for 12 mm carbon fibers is found to be in between 0.10 to 0.15 percent weight contents. The main reason for the reduction of electrical resistivity is the tunnel effect and the lapping of carbon fiber. It is easy to establish the steadier conductive network for more and longer carbon fibers in the cement mortar.
4. The analytical results pertaining to the critical pullout length and percolation threshold using different theoretical approaches are well in

agreement with the experimental findings. The theoretical percolations are bit conservative that might be linked with the complexity in attaining 3-d homogeneous and isotropic dispersion of carbon fibers inside cementitious matrix.

Abbreviations

AR	Aspect ratio
CFs	Carbon fibers
CNTs	Carbon nanotubes
FESEM	Field emission scanning electron microscopy
FRCCs	Fiber reinforced cementitious composites
OPC	Ordinary Portland cement
PP	Polypropylene
PVA	Polyvinyl alcohol
RSFs	Residual strength factors
w/c	Water to cement ratio

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